

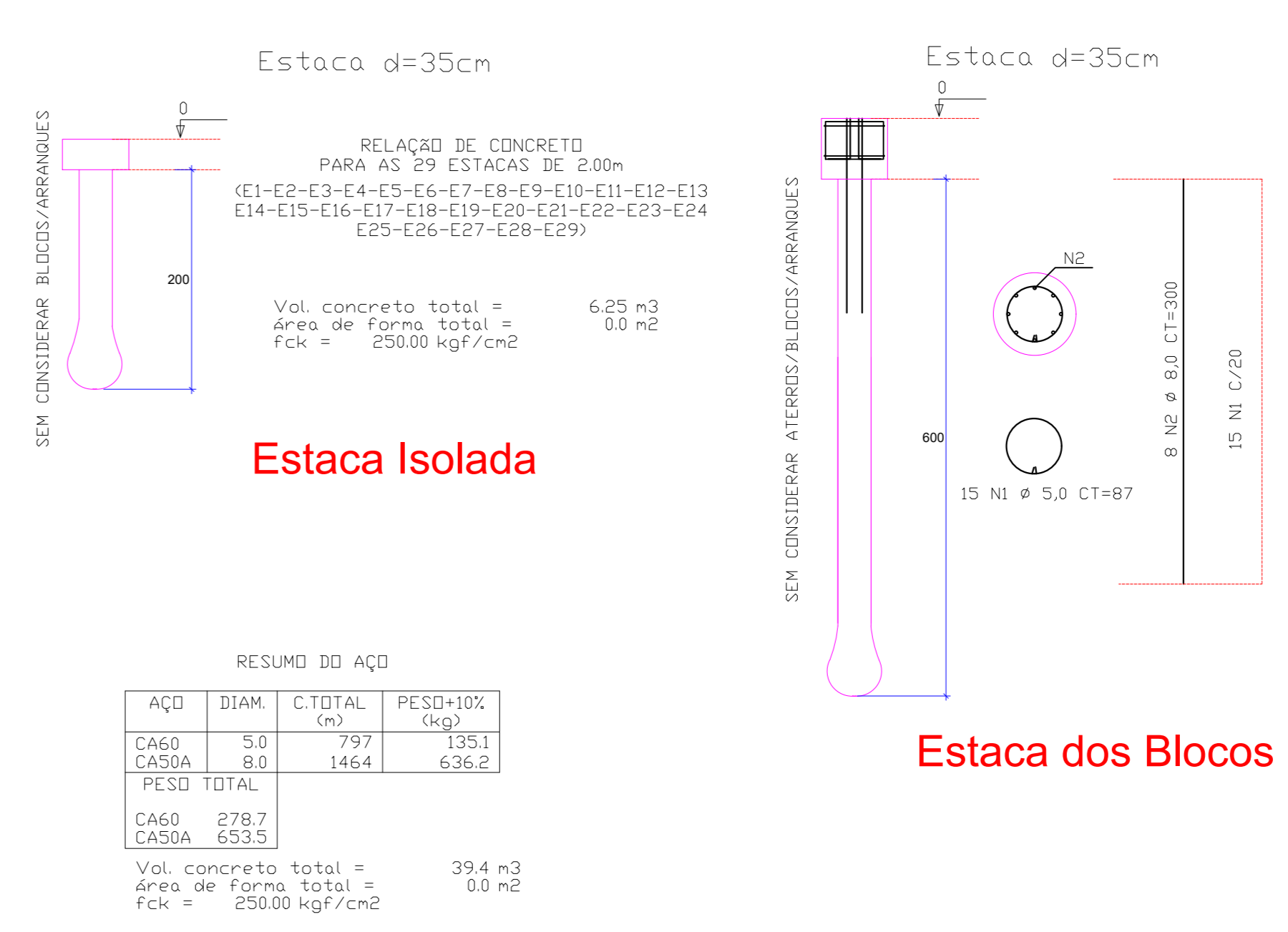
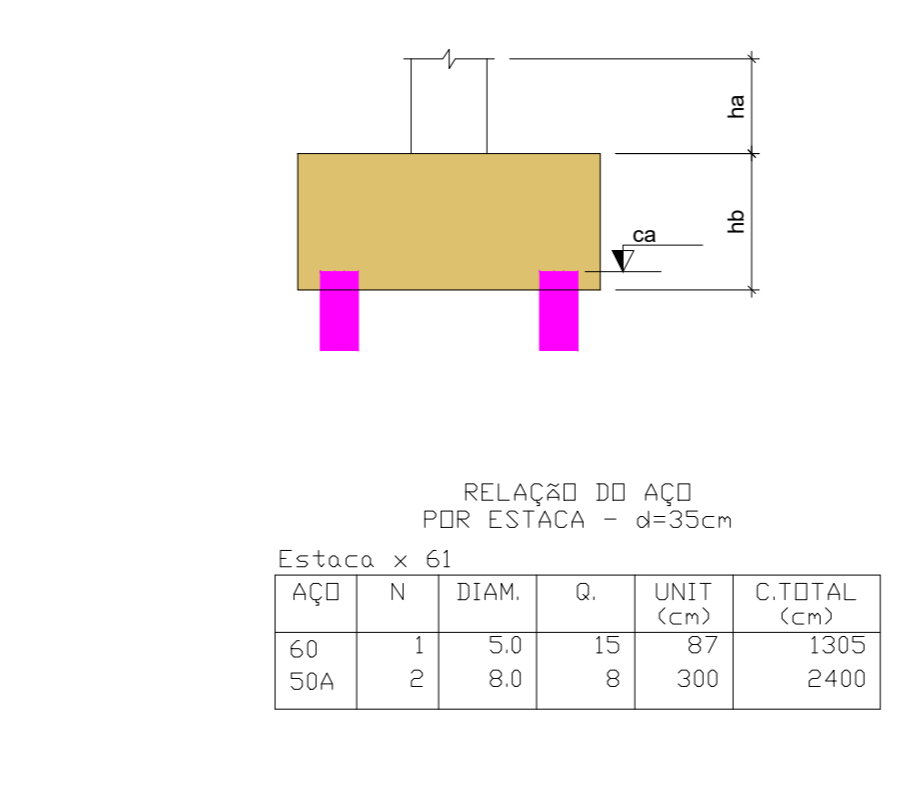
- NOTAS:
1. TODAS AS MEDIDAS DEVEM SER CONFERIDAS NO LOCAL DA OBRA.
 2. VERIFICAR COM PROJETA DA ESTRUTURA O AUMENTO DO COMPRIMENTO DO PILAR E O COMPRIMENTO DE ANCORAGEM DOS PILARES
 3. USAR ESPALHADORES OU PASTILHAS PARA GARANTIR O COBRIMENTO
 4. O FABRICANTE DAS LAJES TRILHÇADAS DEVERÁ FORNECER O PROJETO COMPLETO E A ART DE PROJETO, ASSIM COMO O CERTIFICADO DA PRODUÇÃO DAS TRILHÇAS
 5. AS ARMADURAS DE PELE DEVEM SER FEITAS EM GAIOLAS PARA FIXAÇÃO NA ARMADURA DAS VIGAS

Planta de locação das estacas

LOCAÇÃO DAS ESTACAS										
Bloco	Nome	Coordenada X (m)	Coordenada Y (m)	Carga máx. (kgf)	Carga mín. (kgf)	Momento máx. (kgf.m)	Momento mín. (kgf.m)	Força horiz. máx. (kgf)	Força horiz. mín. (kgf)	CA (cm)
B1	E1-1	403.50	56.50	8.28	2.10	0.00	0.00	0.00	0.00	45
B1	E1-2	487.50	56.50	8.41	2.36	0.00	0.00	0.00	0.00	45
B1	E1-3	403.50	-27.50	8.34	1.14	0.00	0.00	0.00	0.00	45
B1	E1-4	487.50	-27.50	8.12	1.27	0.00	0.00	0.00	0.00	45
B2	E2-1	903.50	56.50	8.12	2.32	0.00	0.00	0.00	0.00	45
B2	E2-2	987.50	56.50	8.24	2.41	0.00	0.00	0.00	0.00	45
B2	E2-3	903.50	-27.50	8.81	1.38	0.00	0.00	0.00	0.00	45
B2	E2-4	987.50	-27.50	8.57	0.90	0.00	0.00	0.00	0.00	45
B3	E3-1	1176.00	-10.50	3.88	1.84	149.14	9.07	0.14	0.02	45
B3	E3-2	260.00	-10.50	4.54	2.98	149.14	9.07	0.14	0.02	45
B4	E4-1	1111.00	-10.50	7.57	5.89	244.70	41.88	0.15	0.02	45
B4	E4-2	48.50	-363.00	9.81	5.43	262.00	0.07	1.10	0.54	40
B5	E5-1	115.00	-363.00	2.38	0.92	262.00	0.07	1.10	0.54	40
B6	E6-1	693.00	-363.00	10.82	7.26	145.55	4.07	0.18	0.01	45
B6	E6-2	737.00	-363.00	11.01	7.37	145.55	4.07	0.18	0.01	45
B7	E7-1	1374.50	-363.00	2.82	0.86	333.25	0.05	1.00	0.48	40
B7	E7-2	1458.50	-363.00	9.82	5.82	333.25	0.05	1.00	0.48	40
B8	E8-1	319.50	-742.00	8.04	3.72	517.98	278.45	0.43	0.28	45
B8	E8-2	39.50	-742.00	7.97	3.61	517.98	278.45	0.43	0.28	45
B9	E9-1	653.00	-717.75	7.45	4.61	0.00	0.00	0.16	0.01	45
B9	E9-2	717.00	-717.75	7.12	4.27	0.00	0.00	0.16	0.01	45
B9	E9-3	685.00	-790.50	7.18	4.73	0.00	0.00	0.16	0.01	45
B10	E10-1	1397.50	-1136.50	8.40	7.94	333.04	1.51	0.31	0.06	45
B10	E10-2	1471.50	-1136.50	4.89	2.99	333.04	1.51	0.31	0.06	45
B10	E10-3	1411.50	-1136.50	6.80	3.92	289.21	9.06	0.27	0.08	45
B10	E10-4	1374.50	-1136.50	5.42	3.86	289.21	9.06	0.27	0.08	45
B12	E12-1	405.50	-1094.50	6.98	4.17	0.00	0.00	0.11	0.01	45
B12	E12-2	489.50	-1094.50	7.18	4.19	0.00	0.00	0.11	0.01	45
B12	E12-3	405.50	-1178.50	8.44	5.52	0.00	0.00	0.11	0.01	45
B12	E12-4	489.50	-1178.50	8.63	5.55	0.00	0.00	0.11	0.01	45
B13	E13-1	869.75	-1094.50	3.97	0.90	0.00	0.00	0.73	0.10	40
B13	E13-2	897.50	-1094.50	10.22	3.30	0.00	0.00	0.73	0.10	40
B13	E13-3	869.75	-1178.50	5.68	4.15	216.52	21.54	0.15	0.03	45
B14	E14-1	1387.50	-1136.50	4.97	4.15	314.20	0.82	0.13	0.01	45
B14	E14-2	1471.50	-1136.50	5.12	3.84	314.20	0.82	0.13	0.01	45
B15	E15-1	31.50	-1514.50	5.46	4.15	216.52	21.54	0.15	0.03	45
B15	E15-2	31.50	-1514.50	5.49	4.47	216.52	21.54	0.15	0.03	45
B15	E15-3	101.50	-1514.50	7.48	158.72	1.91	0.19	0.19	0.03	45
B16	E16-1	502.50	-1522.50	7.46	4.05	158.72	1.91	0.19	0.10	45

LOCAÇÃO DAS ESTACAS										
Bloco	Nome	Coordenada X (m)	Coordenada Y (m)	Carga máx. (kgf)	Carga mín. (kgf)	Momento máx. (kgf.m)	Momento mín. (kgf.m)	Força horiz. máx. (kgf)	Força horiz. mín. (kgf)	CA (cm)
B17	E17-1	903.50	-1522.50	5.02	3.15	120.81	1.72	0.10	0.00	45
B17	E17-2	987.50	-1522.50	5.01	3.03	120.81	1.72	0.10	0.00	45
B18	E18-1	1429.50	-1558.50	4.47	2.38	108.79	10.05	0.49	0.09	45
B18	E18-2	1429.50	-1558.50	3.74	0.80	108.79	10.05	0.49	0.09	45
B19	E19-1	16.50	-1958.50	8.66	6.74	322.12	33.43	0.15	0.09	45
B20	E20-1	460.50	-1958.50	7.51	5.63	392.24	134.91	0.31	0.15	45
B21	E21-1	2293.50	-2367.50	2.33	1.15	798.71	40.10	0.31	0.01	45
B22	E22-1	2265.50	-2367.50	2.21	1.27	1898.68	74.83	0.40	0.02	45
B23	E23-1	2908.50	-2367.50	2.28	1.19	772.68	62.22	0.30	0.04	45
B24	E24-1	3223.50	-2367.50	2.33	1.23	769.51	61.14	0.30	0.03	45
B25	E25-1	3359.50	-2367.50	2.74	1.79	794.97	71.18	0.31	0.01	45
B26	E26-1	3729.50	-2367.50	2.76	1.81	791.50	67.47	0.31	0.01	45
B27	E27-1	331.50	-2499.50	2.53	1.64	262.68	14.53	0.07	0.01	45
B27	E27-2	52.50	-2499.50	3.33	2.42	262.68	14.53	0.07	0.01	45
B28	E28-1	418.50	-2499.50	2.70	1.18	367.73	57.79	0.18	0.08	45
B28	E28-2	602.50	-2499.50	3.62	2.74	367.73	57.79	0.18	0.08	45
B29	E29-1	2293.50	-2501.50	2.22	1.08	839.25	70.07	0.33	0.03	45
B30	E30-1	2265.50	-2501.50	2.24	1.26	809.81	77.69	0.41	0.07	45
B31	E31-1	2908.50	-2501.50	2.28	1.19	772.68	62.22	0.30	0.04	45
B32	E32-1	3223.50	-2501.50	2.33	1.23	769.51	61.14	0.30	0.03	45
B33	E33-1	3359.50	-2501.50	2.55	1.69	797.24	73.77	0.31	0.01	45
B34	E34-1	3729.50	-2501.50	2.56	1.67	794.67	69.87	0.31	0.01	45
E1	E1-1	16.50	-1958.50	0.70	0.88	691.83	941.21	0.64	0.62	0
E2	E2-1	460.50	-1958.50	1.78	1.65	204.68	12.46	0.18	0.02	0
E3	E3-1	1379.50	-1958.50	0.64	0.65	1006.61	971.49	0.68	0.64	0
E4	E4-1	16.50	-2025.50	3.59	3.58	125.31	69.05	0.92	0.90	0
E5	E5-1	1379.50	-2025.50	3.59	3.58	107.34	35.52	0.90	0.88	0
E6	E6-1	2293.50	-1136.50	2.62	2.48	317.71	226.48	0.54	0.41	0
E7	E7-1	2293.50	-1136.50	1.94	1.86	142.31	14.31	0.10	0.01	0
E8	E8-1	1186.50	-1136.50	2.44	2.42	316.53	215.16	0.45	0.39	0
E9	E9-1	16.50	-1329.50	2.85	2.55	676.82	110.86	0.80	0.39	0
E10	E10-1	3729.50	-1329.50	2.96	1.95	36.61	48.48	0.14	0.11	0
E11	E11-1	2293.50	-1522.50	1.89	2.02	199.49	40.61	0.21	0.09	0
E12	E12-1	2293.50	-1522.50	1.86	1.48	348.13	33.03	0.38	0.02	0
E13	E13-1	1186.50	-1522.50	1.89	2.42	448.24	226.48	0.46	0.39	0
E14	E14-1	16.50	-1758.50	1.56	1.72	340.02	0.63	0.27	0.04	0
E15	E15-1	321.50	-1758.50	1.18	1.15	88.50	35.15	0.12	0.07	0
E16	E16-1	460.50	-1758.50	1.73	1.59	253.84	24.95	0.19	0.03	0
E17	E17-1	321.50	-1958.50	2.68	2.53	240.54	30.61	0.25	0.06	0

Estacas			
Simbologia	Nome	Quantidade	Profundidade (de fund. em)
	C35 35.00	61	6.00
	C35 35.00	29	2.00



PROJETO ESTRUTURAL

ENDEREÇO: TRINDEADO - GO

PROPRIETÁRIO: VILA SÃO JOSÉ BENTO COTTELONGO

AUTOR DO PROJETO: ENG. STEFANY PACHECO DE FARIA

RESPONSÁVEL TÉCNICO: [Blank]

ENGENHARIA: MS PROJETA

REVISÃO: [Blank]

DISCIPLINA: PROJETO ESTRUTURAL

CONTEÚDO: Planta de Locação das Estacas

DATA: 27/07/2020

PRANCHA: 02/15

REVISÃO: R00

RESPONSÁVEL: MATEUS HENRIQUE

RESUMO DO AÇO:

ACQ	N	DIAM.	Q	UNID.	C.TOTAL (CM)
CA60	5.0	7.97	1351	636.2	
CA60	8.0	1464	1351	636.2	
PESO TOTAL:					

Vol. concreto total = Área de forma total = 39.4 m³ / 0.0 m³

Fck = 25000 kgf/cm²